



FRACTURE ENERGY OF FOAMED CONCRETE BY MEANS OF THE THREE-POINT BENDING TESTS ON NOTCHED BEAM SPECIMENS

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ABSTRACT

Foamed concrete has recently gained attention as an alternative to normal concrete in structural engineering. Its low range of densities with good strength, high serviceability and lightness that lead to the many applications. In the last few years, many studies have been conducted to determine the mechanical properties and strengths of foamed concrete, especially high-performance characteristics. However, these studies did not examine the fracture energy, which is the important parameter of structural behaviour and damage mechanisms. Fracture energy represents the ability of a structural element to resist crack propagations during the softening phase. This study, therefore, aims to experimentally investigate the fracture energy of foamed concrete. The notched beam specimens of foamed concrete were prepared at densities of 1400kg/m³ and 1600kg/m³. The dimension of beam specimen is 700mm × 150mm × 150mm. Two types of notches; U and V shapes were used. The notch has a depth of 30mm and located at the mid-span of beam specimens. Meanwhile, cube samples were also cast to access the compressive strength. All notched beam specimens were tested using the three-point bending test to obtain the load-deflection profiles. The results were used to calculate fracture energy using Hillerborg model. Meanwhile, the fracture energy was also calculated using Bazant, CEB and Oh models. A comparison of these models shows a favourable agreement with the fracture energy of foamed concrete is in the range of 18N/m to 25N/m. surprisingly, the fracture energy is relatively high for compression strength of foamed concrete at a range of 6.4MPa to 14.7MPa.

Keywords: fracture energy, foamed concrete, notched beam specimen, three-point bending test.

INTRODUCTION

It is cogently undeniable that foamed concrete has become the most popular lightweight material in construction and infrastructural industries. This is due to the advantages offered by foamed concrete such as low density, sufficient durability, excellent fire resistance and good thermal conductivity. With its promising density and strength, foamed concrete is a viable solution for reducing loading, an especially important issue that needs to be tackled in the soft soil area. Typical densities of foamed concrete are ranged around 1000kg/m³ to 1600kg/m³ (Rammurty *et al.*, 2009) while the compressive strength can achieve up to 12MPa. In addition, high-performance foamed concrete has been introduced by many researchers such as Liu and Jiang (2012), Bing *et al.* (2012) and Hilal *et al.* (2015) to surpass conventional concrete. However, the greatest invention was produced by Just and Middenorf (2009) that propose a high-performance foamed concrete with strength up to 105.7MPa using aluminium powder, superplasticizer and microsilica. This type of foamed concrete can be utilised as cast-in-place beams and columns, load bearing walls, sandwich panels, prestressed structures and refractories.

High renewed interests on the application of foamed concrete as structural elements have lead to various investigations on the engineering properties, strengths and structural behaviours of foamed concrete. Many experimental studies were conducted to determine the compression, tensile and flexural strengths. These

strengths are associated with the performance and durability of the structure. In terms of serviceability and crack resistance, fracture energy is the most important parameter that governs the cracking and failure of the structure. It also represents the toughness of the material. Until recently, the investigate on fracture energy is still limited to normal concrete, mortar and lightweight aggregate concrete. Early study by Wang and Li (2003) suggested that the fracture energy of foamed concrete is only a fraction of that obtained from normal concrete. Abd Rahman *et al.* (2015) reviewed extensive test on foamed concrete and concluded that the fracture energy is around 18N/m to 25N/m.

Since there are huge demands about the application of foamed concrete as material for structural elements, but lack of knowledge of its fracture energy, this study intends to examine in-depth the fracture energy of foamed concrete using experimental approach. The establishment of specifications and procedures in the investigation of fracture energy of foamed concrete is based on the current standard that practically applied for mortar and normal concrete. This study contributes to a rational prediction of fracture energy of foamed concrete that later can be implemented in the analysis, design and numerical modelling of structural elements.



FRACTURE ENERGY

In general, fracture energy is defined as the total amount of work required to break a specimen completely per unit ligament area (Li *et al.*, 1995; Jirasek *et al.*, 2004). It can also be referred to as energy required in the incarnation and the formation of cracks as dissipated per unit area. Basically, fracture energy corresponds to the area of the softening phase at a stress-strain curve. Hillerborg (1985) suggested that fracture energy can be simply determined using Equation (1) below:

$$G_f = \frac{U_o + m_g d_o}{B(W - a_o)} \quad (1)$$

where: U_o is area under the softening phase of the stress-strain curve that obtained from bending test, m_g is specimen weight, d_o is load-point deflection at the fracture, B is specimen width, W is specimen depth, and a_o is notch depth.

Meanwhile, Comite Euro-International du Beton, CEB (1993) proposed a simple empirical formula to determine the fracture energy. The formula as depicted in Equation (2) is based on two main factors known as compressive strength and aggregate size. Similarly, Bazant (2002) introduced the formula as in Equation (3) that include water-cement ratio as an additional parameter.

$$G_F = \left[0.0469(D_{max})^2 - 0.5D_{max} + 26 \right] \times \left(\frac{f_c}{10} \right)^{0.7} \quad (2)$$

$$G_F = 2.5\alpha_0 \left(\frac{f_c}{0.058} \right)^{0.40} \left(1 + \frac{D_{max}}{1.94} \right)^{0.43} \left(\frac{w}{c} \right)^{-0.18} \quad (3)$$

where: α_0 is an aggregate shape factor where 1 for the rounded aggregate and 1.12 for the angular aggregate, D_{max} is the maximum aggregate size, f_c is the compressive strength and w/c is the water-cement ratio of concrete.

Martin *et al.* (2007) and Arezoumandi *et al.* (2014) performed a comparative study using these three models and found that Bazant and CEB models showed excellent agreement with the Hillerborg test data. A recent study by Abd Rahman *et al.* (2015) also found that the fracture energy of foamed concrete from Hillerborg model falling within 10% of the predicted by Bazant and CEB models. Inspired by tensile strength and modulus of elasticity, a rather simple formula was proposed by Bazant and Oh (1983):

$$G_F = (15.53 + 17.72f_t) f_t^2 \left(\frac{d_a}{E_c} \right) \quad (4)$$

On the other hand, Oh *et al.* (1999) simplified the equation as follows:

$$G_F = \frac{54.24f_t d_a}{E_c} \quad (5)$$

where: f_t is the tensile strength of concrete, d_a is the diameter of aggregate and E_c is the modulus of elasticity. When the values of f_t and E_c of foamed concrete are not measured directly through experiment, the usual relations as suggested by Byun *et al.* (1998), Jones and McCarthy (2005), Ramamurthy and Nambiar (2009) and Jaini *et al.* (2015) may be used, which read:

$$E_c = 0.42f_c^{1.18} \quad (6)$$

$$f_t = 1.03(f_c)^{0.5} \quad (7)$$

For mortar and normal concrete, f_t and E_c can be calculated based on a recommendation by Eurocode 2 (2004). However, it is noticeable that Equations (4) and (5) may not suitable to be used for foamed concrete. Likely due to its empirical formula may mislead the value of tensile strength and the prediction of Young's modulus. In addition, Rahman and Jaini (2013) used this formula to predict the fracture energy of foamed concrete and the value is 40% underestimated the actual fracture energy.

HILLERBORG MODEL

Hillerborg model or also known as the work-of-fracture method is the most commonly and widely used to determine the fracture energy of brittle materials. Fracture energy is computed as the area under the experimental load-deflection response for a notched beam specimen subjected to a point load. Figure-1 shows a graphical representation of this concept. Since introduced by Hillerborg (1985), many other researchers have implemented a similar technique to determine the fracture energy of concrete, as example Malvar and Warren (1988), Abdalla and Karihaloo (2002), and Antico *et al.* (2011). Moreover, the Hillerborg model was also adopted in a standard test method such as RILEM (1990), NT Build 491 (1999) and ASTM E1820-13 (2014). Early in the adoption of this model, one issue of interest is the impact of size effects on fracture energy, especially the influence of dimensions, geometries and notch conditions. Shah *et al.* (1995) investigated the size effects on fracture energy and found that fracture energy exhibit considerable dependence on beam specimen size. Hanson and Ingraffea (2003) found that fracture energy increases with increasing the depth of beam specimen. Sim *et al.* (2014) stated that the notch is a more critical parameter than the depth of beam specimen. It is should be considered here that fracture energy also depends on the size of aggregate (Elices and Rocco, 2008).

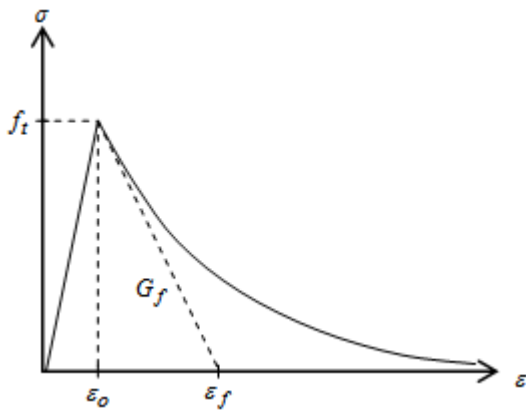


Figure-1. Fracture energy under the stress-strain curve.

According to Prasad and Sagar (2008), size effects may be defined by considering geometrically similar structures or specimens of different sizes, where the nominal strength at the failure stage is taken as:

$$\sigma_N = c_N \frac{P_u}{td} \tag{8}$$

where: c_N is arbitrary constant, P_u is maximum load, t is thickness and d is a characteristic dimension of the beam specimen. Hu and Wittman (2000) and Cifuentes *et al.* (2013) proposed a method of measuring the size independent fracture energy of concrete. Meanwhile, Lee and Lopez (2014) proposed notched beam specimen with counterweight as experimental correction to the current practice in determining the fracture energy.

EXPERIMENTAL DETAILS

An experimental study to determine the fracture energy of foamed concrete was mainly conducted using the Hillerborg model. Unlike normal concrete and mortar, there are no details of specifications and procedures that can be referred to conduct the experimental study. Therefore, the current standard test method was adopted.

Specimen design

The design of notched beam specimens is based on two standard test methods of RILEM (1990) and ASTM E1820-13 (2014). The effective span to depth ratio of beam specimen was setup as $S/W=4$. Hence, the dimension of beam specimen is $700\text{mm} \times 150\text{mm} \times 150\text{mm}$. The depth ratio of the notch was taken as $a_0/W=0.2$ as suggested by Oh *et al.* (1997), hence, the depth of the notch is $a_0=30\text{mm}$. Two types of U and V-notch were used and positioned at the center of beam specimen. The specifications of beam specimen can be referred in Figure-2.

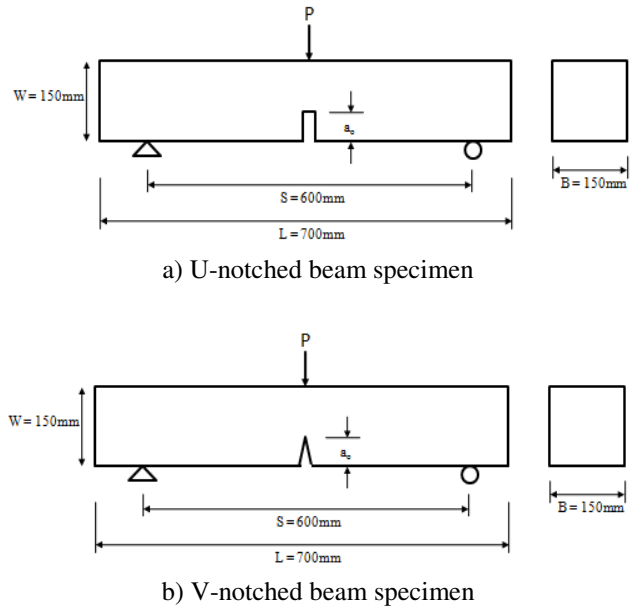


Figure-2. Schematic design of the beam specimens, where depth and width of beam, B and W= 150mm, span of beam, S= 700mm and depth of notch, $a_0=30\text{mm}$.

In this experimental study, a total of 16 notched beam specimens were prepared for the three-point bending test. It means that each type of concrete has two U-notched beam specimens and another two of V-notched beam specimens. Table-1 shows the quantity of beam specimens.

Table-1. Specimen preparation for the three-point bending test

Type of Concrete	Density (kg/m ³)	Number of specimen	
		U-Notch	V-Notch
Foamed (FC1400)	1400	2	2
Foamed (FC1600)	1400	2	2
Mortar (MR2000)	2000	2	2
Normal (NC2400)	2000	2	2
Total of specimen =		8	8

Mix design and materials

The mix design based on the Department of Environment (DOE) was utilized to evaluate the proportions of foamed concrete, mortar and normal concrete. The proposed densities of foamed concrete are 1400 kg/m^3 and 1600 kg/m^3 . Meanwhile, mortar and normal concrete have densities of 2000 kg/m^3 and 2400 kg/m^3 , respectively. In order to achieve the targeted compressive strength, especially for foamed concrete, the



cement-sand ratio, water-cement ratio and foam-cement ratio were fixed according the requirement. The concentration of foam agent is 140kg/liter. Fine and coarse aggregates have maximum diameters of 3mm and 20mm

respectively. Table-2 shows the mix design of foamed concrete, mortar and normal concrete. This study was used the Type 1 of Ordinary Portland Cement (OPC) for all types of concrete.

Table-2. Mixed design of foamed concrete, mortar and normal concrete.

Mixture	FC1400	FC1600	MR2000	NC2400
Cement-Sand ratio (C/S)	0.5	0.5	0.5	0.5
Foamed-Cement ratio (F/C)	0.7	0.7	-	-
Water-Cement ratio (W/C)	0.55	0.55	0.5	0.5
Cement (kg)	26	32	40.5	450
Fine aggregate (kg)	50	64	81	930
Coarse aggregate (kg)	-	-	-	968
Water (liter)	15	17.6	20.25	225
Foam (liter)	26	22.4	-	-

Compression test

Cube samples with size of 150mm×150mm×150mm were casting for the compressive test. Each type of concrete was allocated with six cube samples. The cube samples were placed under compressive loading after 7, 14 and 28 days long curing process to determine the compressive strength. All cube samples were cured using air curing techniques at the ambient temperature. Figure-3 depicts the compressive test on the cube samples.

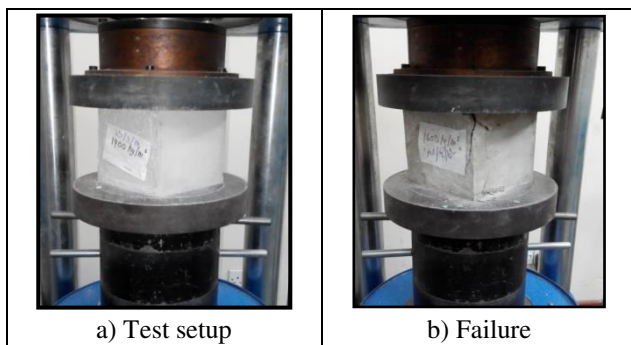


Figure-3. Compression test for cube samples.

Three-Point bending test

The three-point bending test was conducted for all beam specimens after 28 days curing period. The universal testing machine was setup to the minimum speed at 0.05mm/min to provide sufficient time during which the propagation of the crack can be detected and to prevent sudden catastrophic failure. The rate of loading should be maintained until failure of beam specimens. Both supports were hinged with rollers. The beam specimens must be handled carefully during the placement on the testing

machine to avoid pre-crack growth. The load, deflection and crack formation of beam specimens were recorded to plot the load-deflection profile and crack-load history. Figures 3 and 4 illustrate the test setup for both U-notched and V-notched beam specimens. In this experimental study, the load and deflection were measured directly from the universal testing machine. The stroke of universal testing machine was calibrated with the linear variable displacement transducer (LVDT) to ensure the validity of data.

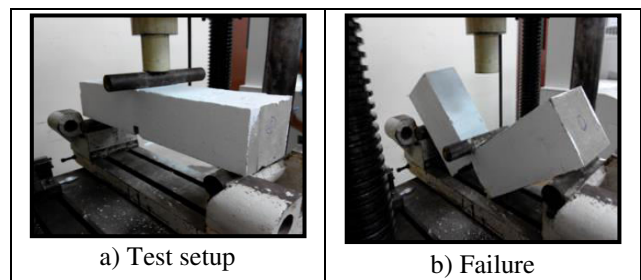


Figure-4. Three-point bending test for U-notched beam specimen.

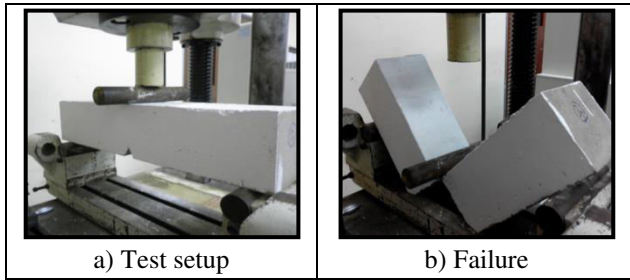


Figure-5. Three point bending test for V-notched beam specimen.

EXPERIMENTAL RESULTS

Compressive strengths and load-deflection profiles were obtained from compression test and three-point bending test respectively. These experimental results are paramount important for prediction of fracture energy. A comparison of fracture energy, calculated using a Hillerborg model with Bazant, CEB and Oh models were performed to appraise the results from the three-point bending test.

Compressive strength

Compressive strength of foamed concrete, mortar and normal concrete at 7, 14 and 28 days is presented in Figure-6. The strength of foamed concrete at densities of 1400kg/m^3 and 1600kg/m^3 has shown a standard compressive strength at 6.4MPa and 14.7MPa respectively. The compressive strengths of foamed concrete are considerable accepted as suggested by Aldrige (2000) and the British Cement Association (1994) where the strength should in the range of 6.0MPa to 8.0MPa for density 1400kg/m^3 and 7.5MPa to 10.0MPa for density 1600kg/m^3 . Meanwhile, mortar and normal concrete achieved the respective target characteristic strength of 25MPa. The compressive strength of mortar at a density of 2000kg/m^3 is 23.5MPa while normal concrete is 26.3MPa for density of 2400kg/m^3 .

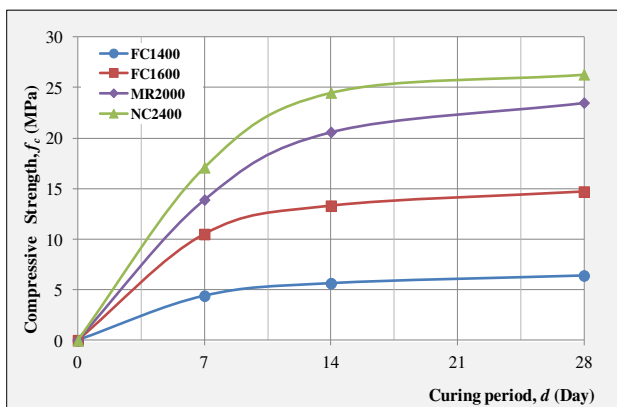


Figure-6. Compressive strengths of the cube specimens for foamed concrete (FC1400 and FC1600), mortar (MR2000) and normal concrete (NC2400).

Load-Deflection profile

In Hillerborg model, the load-deflection profile is the important parameters that need to be determined. The load-deflection profiles for U-notched and V-notched beam specimens are displayed in Figures 7 and 8. It was found that the ultimate load of U-notched and V-notched beam specimens are different, where U-notched beam specimens produce significantly high value compared to V-notched beam specimens. This indicates that high stress occurs at the narrow point of V-notch and leads to the faster formation of cracks. However, the total area under the load-deflection profiles produced by V-notched beam specimens are slightly larger. Unlike normal concrete and mortar that experience catastrophic failure, foamed concrete shows complete softening phase. The residual strength due to the complex microstructure allow it to sustain loading until the beam specimens broken into two halves.

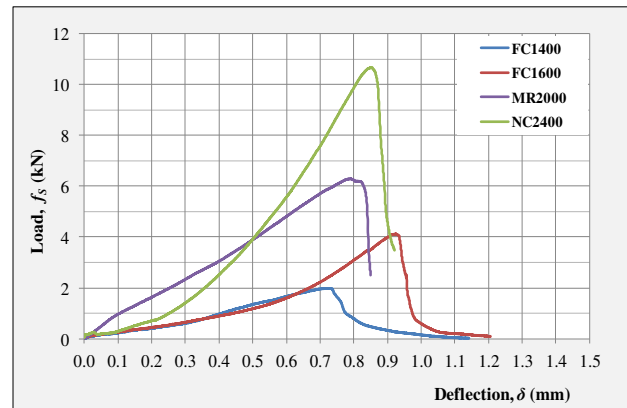


Figure-7. Load-deflection profiles of the U-notched beam specimens for foamed concrete (FC1400 and FC1600), mortar (MR2000) and normal concrete (NC2400).

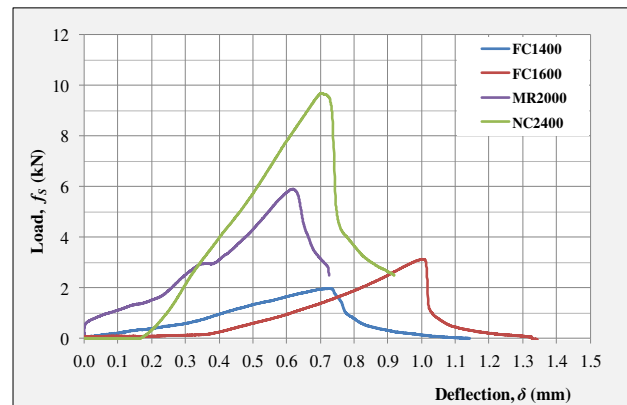


Figure-8. Load-deflection profiles of the V-notched beam specimens for foamed concrete (FC1400 and FC1600), mortar (MR2000) and normal concrete (NC2400).



Fracture energy

The determination of fracture energy using Hillerborg model requires parameters such as total area under load-deflection profile, weight of beam specimen, dimension of beam and the depth of notch. Meanwhile, compressive strength becomes an important parameter for Bazant and CEB models. Oh model requires tensile strength and Young's modulus that can be determined by experimental study or empirical formula. The data of compressive strength, ultimate strength and maximum deflection for all types of concrete are shown in Table-3. The values of fracture energy were calculated using Equations (1) to (3) and Equation (5). Table-4 shows the fracture energy for all types of concrete using U-notched and V-notched beam specimens.

Using Hillerborg model, the prediction of fracture energy for U-notched and V-notched beam specimens are

almost identical, especially for foamed concrete and mortar. Fracture energy using V-notched beam specimens gave slightly higher value around 15% to 25% for foamed concrete and mortar, while approximately 32% for normal concrete. The correspondence of fracture energy proof that the type of notch plays insignificant role. In view that the strength of foamed concrete is mostly governed by the amount of sand, sand-cement ratio and particle size distribution of sand as stated by Ravindra *et al.* (2005), hence, the fracture energy of foamed concrete is lower than mortar and normal concrete. The present of voids on foamed concrete also reduces the strength, consequently affected the crack resistance. Although the fracture energy of foamed concrete is only around 18N/m to 25N/m, it indicates relatively high level despite lower compressive strength.

Table-3. Parameter of strengths and maximum deflection.

Type of concrete	Density (kg/m ³)	f_c (MPa)		f_s (kN)		δ_{max} (mm)	
		U	V	U	V	U	V
Foamed	1400	6.40	6.40	10.60	9.66	0.85	0.72
Foamed	1600	14.70	14.70	6.30	5.90	0.79	0.62
Mortar	2000	23.50	23.50	4.14	3.13	0.92	1.00
Normal	2400	26.30	26.30	1.98	1.97	0.72	0.73

Table-4. Fracture energy of foamed concrete, mortar and normal concrete using U-notched and V-notched beam specimens.

Type of concrete	Density (kg/m ³)	Fracture energy (N/m)				
		Hillerborg		Bazant	CEB	Oh
		U	V			
Foamed	1400	18.20	18.13	18.28	19.02	26.41
Foamed	1600	20.44	25.35	25.50	34.05	15.00
Mortar	2000	35.96	31.12	31.30	47.28	12.90
Normal	2400	63.13	83.36	32.86	51.14	90.39

Referring to Hillerborg model, the fracture energy of mortar is in the range of 31N/m to 35N/m. This fracture energy is close to that suggested by Wittmann (2002). On the other hand, normal concrete has fracture energy around 75N/m to 100N/m (Muralidhara, 2011). Results from experimental study revealed a favourable agreement. Despite the mortar has almost similar compressive strength with normal concrete, but the fracture energy is very noticeably low. According to Darwin *et al.* (2001), the size of aggregate is likely influences the fracture energy where the small size of aggregate tends to produce low and minimum fracture energy. Moreover, Ishiguro (2001) found that the fracture

energy of mortar is slightly increased by increasing the compressive strength.

A comparison of fracture energy predicted using a Hillerborg model with Bazant, CEB and Oh models are shown in Figure-9. The fracture energy significantly increase by density. At the lower densities from 1400kg/m³ to 2000kg/m³, Bazant model has provided results at almost exactly as Hillerborg model. However, both Bazant and CEB models underestimated the fracture energy of normal concrete. Although Oh model not perfectly determine the fracture energy of foamed concrete and mortar, but the fracture energy of normal concrete is within the expected range. As mention earlier, Oh model may only suitable for normal concrete that has established



method in predicting the tensile strength and modulus of elasticity.

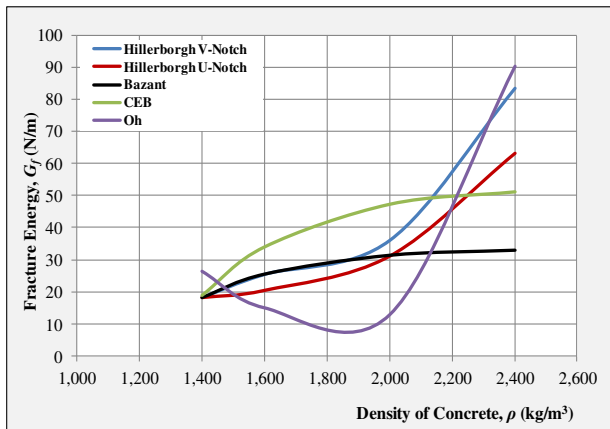


Figure-9. A comparison of fracture energy of foamed concrete, mortar and normal concrete using various models.

CONCLUSIONS

An experimental study was conducted on the fracture energy of foamed concrete using the three-point bending test. This test mainly measured the amount of energy absorbed when the beam specimen broke in half. Compression strengths and load-displacement profiles were obtained and analyzed. The Hillerborg model was used to determine fracture energy, where the area of load-deflection profile and maximum deflection become the predominant parameters. The results were compared with that calculated using Bazant, CEB and Oh models. It is found that the fracture energy of mortar and normal concrete are at acceptable range of 31N/m to 84N/m. This indicates that the specifications and procedures for the three-point bending test were properly designed and conducted. In addition, the fracture energy of foamed concrete ranged from 18N/m to 25N/m. Although, the fracture energy of foamed concrete is a fraction of normal concrete, it is relatively high for compressive strength around 6.4MPa to 14MPa. It is found that the fracture energy significantly increase by density and compressive strength.

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